

# DOOR SYSTEMS INC

## DSI-FW-W119 ELEVATOR SMOKE AND FIRE CURTAIN

DES. J. ROBERSON

JOB NO. 11-2503

DATE 4/30/25

SHEET

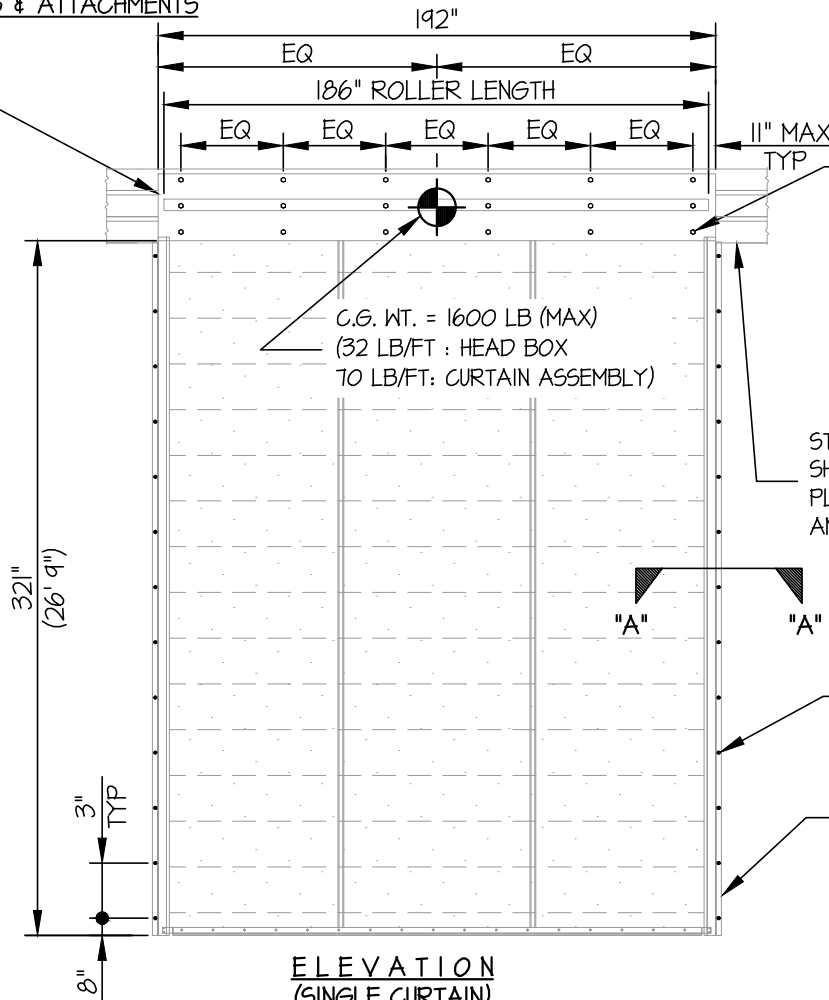
1

OF 7 SHEETS

## SEISMIC SUPPORTS &amp; ATTACHMENTS

WALL MOUNTED

HEADBOX  
(18 GA, A653,  
Fy=30 KSI MIN)  
(BY MFR)



USE #14 TEK SCREWS AT 16" O.C. MAX  
W/ 3/16" x 1/4" FENDER WASHERS  
(TOP, MIDDLE & BOTTOM, 18)  
TO STEEL STUD WALL  
OR  
USE 3/8"  $\phi$  HILTI KB-TZ2 (CS)  
EXPANSION ANCHORS  
(MIN. EMBED. (h<sub>ef</sub>) = 2") @ 36" O.C.  
W/ 1/2" x 1/2" FENDER WASHERS  
(TOP, MIDDLE & BOTTOM)  
TO CONCRETE WALL  
(BY STRUCTURAL ENGINEER OF RECORD)

STRUCTURAL ENGINEER OF RECORD  
SHALL DESIGN THE BACKING  
PLATE (16 GA., 50 KSI MIN.)  
AND THE WALL STRUCTURE

SEE SHEET 7 OF 7

"A"

"A"

#10 TEK SCREWS

SIDE GUIDE  
(11 GA, A653, Fy=30 KSI MIN)  
(BY MFR) (2 TOTAL)

ELEVATION  
(SINGLE CURTAIN)

## NOTES:

- FORCES ARE DETERMINED PER 2022 CALIFORNIA BUILDING CODE AND ASCE 7-16. STRENGTH DESIGN IS USED. (EXAMPLE:  $a_p = 1.0$ ,  $l_p = 1.5$ ,  $R_p = 1.5$ ,  $z/h \leq 1$ )

Sds	1.60	2.00
HORIZONTAL FORCE ( $E_h$ )	1.92 $W_p$	2.40 $W_p$
HORIZONTAL FORCE ( $E_{mh}$ )	3.84 $W_p$	4.80 $W_p$
VERTICAL FORCE ( $E_v$ )	0.32 $W_p$	0.40 $W_p$

- THIS CALCULATION ENCOMPASSES WEIGHTS AND C.G. POSITIONS NOT EXCEEDING VALUES SHOWN.
- THIS CALCULATION WAS PREPARED WITHOUT KNOWLEDGE OF ANY SITE CONDITION. COMPATIBILITY FOR USE WITH A SITE SHALL BE EVALUATED BY THE STRUCTURAL ENGINEER OF RECORD OF THE INSTALLATION (SEOR). USE REQUIRES APPROVAL BY THE SEOR.
- STRUCTURAL ENGINEER OF RECORD FOR THE INSTALLATION SHALL VERIFY ALL CONDITIONS, EVALUATE INTERACTION WITH ADJACENT EQUIPMENT AND ANCHORS, AND PROVIDE SUPPORT STRUCTURE DESIGNED TO SUPPORT WEIGHTS AND FORCES SHOWN IN COMBINATION WITH ALL OTHER LOADS THAT MAY BE PRESENT.



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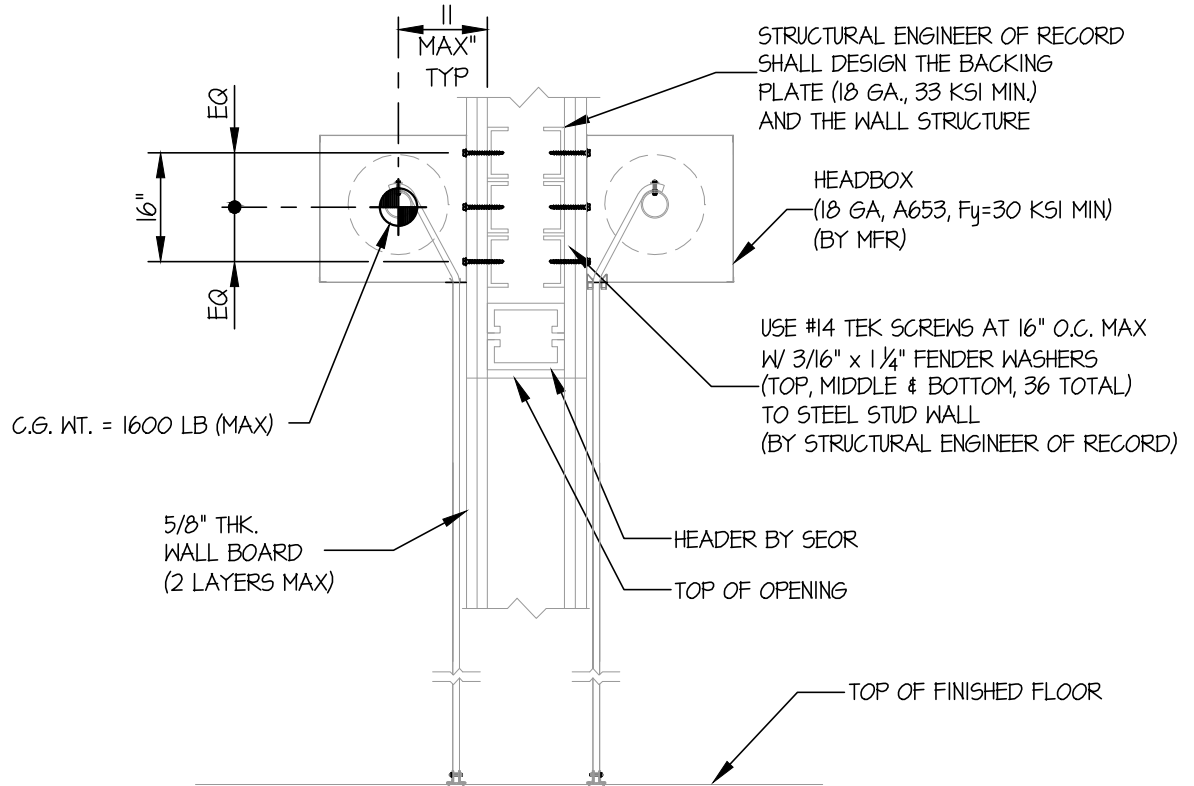
**2**

OF **7** SHEETS

SEISMIC SUPPORTS & ATTACHMENTS

$Sds \leq 1.60$

WALL MOUNTED



SECTION AT STEEL STUD WALL  
(SINGLE CURTAIN)

LOADS:

WEIGHT ( $W_p$ ) = 1600 LB MAX  
HORIZONTAL FORCE ( $E_h$ ) =  $1.92 W_p = 3072$  LB  
VERTICAL FORCE ( $E_v$ ) =  $0.32 W_p = 512$  LB  
(EXAMPLE:  $Sds = 1.60$ ,  $a_p = 1.0$ ,  $l_p = 1.5$ ,  $R_p = 1.5$ ,  $\Omega_o = 2.0$ ,  $z/h = 0$ )

SCREW FORCES:

TENSION (T)

$$T_U \text{ VERTICAL} = \frac{(1.2(1600\#) + 512\#)(11'')}{12 \text{ SCREWS}(16'')} = 140 \text{ LB/SCREW}$$

$$T_U \text{ PARALLEL} = \frac{3072\#(11'')}{6 \text{ SCREWS}(160'')} = 35 \text{ LB/SCREW}$$

$$T_U \text{ PERP} = \frac{3072\#}{36 \text{ SCREWS}} = 85 \text{ LB/SCREW}$$

$$T_U \text{ MAX} = 140\# + 35\#(0.3) + 85\# = 236 \text{ LB/SCREW (MAX)}$$

SHEAR (V)

$$V_U \text{ WALL} = \sqrt{\left(\frac{(1.2(1600\#) + 512\#)}{36 \text{ SCREWS}}\right)^2 + \left(\frac{3072\#}{36 \text{ SCREWS}}\right)^2} = 109 \text{ LB/SCREW (MAX)}$$

#14 TEK SCREWS (16 GA, 50 KSI STEEL STUDS)  
W/ 2 LAYERS GYP BOARD MAX

$\phi T = 418$  LB/SCREW

$\phi V = 266$  LB/SCREW

UNITY CHECK:

$$\left(\frac{T_u}{\phi T}\right) + \left(\frac{V_u}{\phi V}\right) \leq 1.0$$

$$\left(\frac{236}{418}\right) + \left(\frac{109}{266}\right) = 0.98 \leq 1.0 \therefore \text{O.K.}$$

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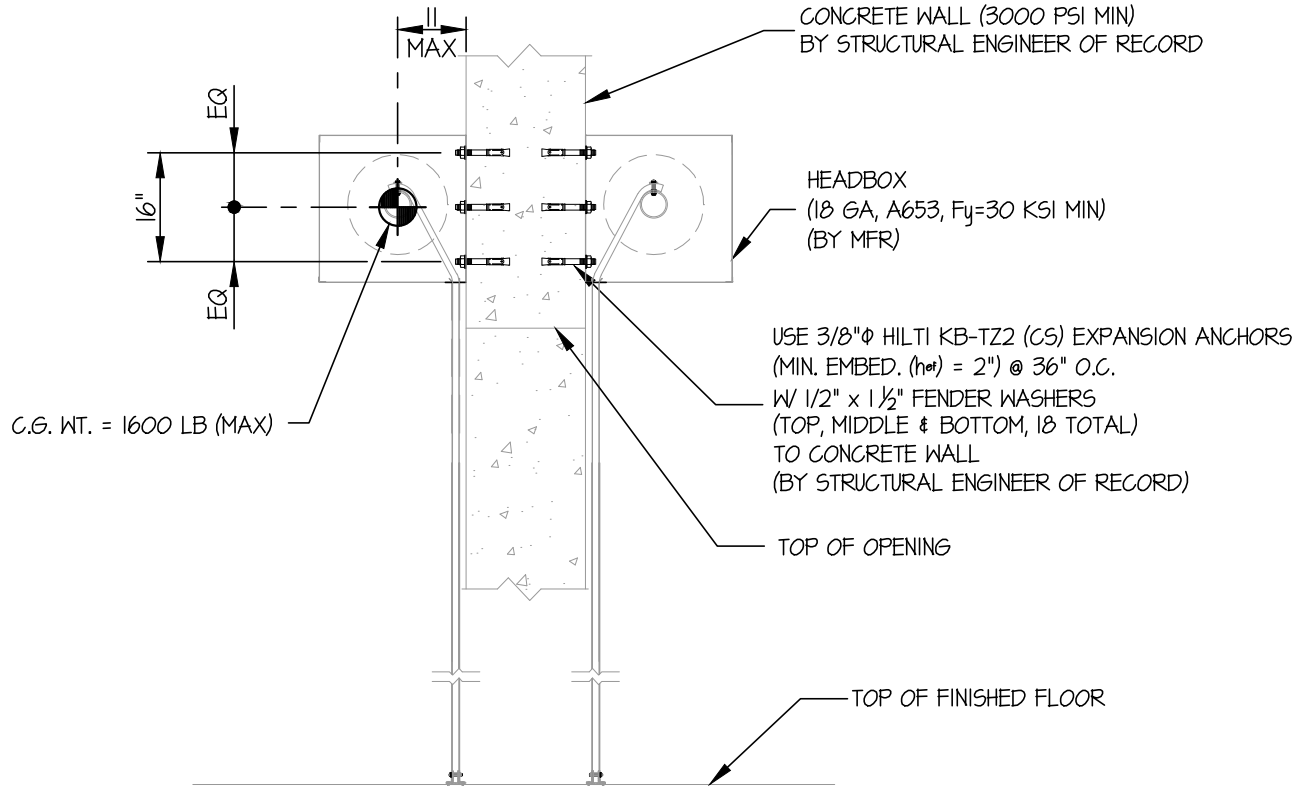
3

OF 7 SHEETS

SEISMIC SUPPORTS & ATTACHMENTS

$S_{DS} \leq 2.00$

WALL MOUNTED



SECTION AT CONCRETE WALL  
(SINGLE CURTAIN)

#### LOADS:

WEIGHT ( $W_p$ ) = 1600 LB MAX  
HORIZONTAL FORCE ( $E_{mh}$ ) =  $4.80 W_p = 7680$  LB  
VERTICAL FORCE ( $E_v$ ) =  $0.40 W_p = 640$  LB  
(EXAMPLE:  $S_{DS} = 2.00$ ,  $a_p = 1.0$ ,  $l_p = 15$ ,  $R_p = 15$ ,  $\Omega_0 = 2.0$ ,  $z/h = 0$ )

ANCHOR SPEC: 3/8"  $\phi$  HILTI KB-TZ2 (CS); ( $h_{ef} = 2"$ )  
SPACING = 6" MIN  
EDGE DISTANCE = 14" MIN:

$\phi T = 0.75 \phi N_h = 1586$  LB/ANCHOR (TENSION)  
 $\phi V = \phi V_h = 2201$  LB/ANCHOR (SHEAR)

#### SCREW FORCES:

TENSION (T)

$$T_U \text{ VERTICAL} = \frac{(1.2(1600\#) + 640\#)(11")}{6 \text{ BOLTS } (16")} = 293 \text{ LB/BOLT}$$

$$T_U \text{ PARALLEL} = \frac{7680\#(11")}{3 \text{ BOLTS } (180")} = 157 \text{ LB/BOLT}$$

$$T_U \text{ PERP} = \frac{7680\#}{18 \text{ BOLTS}} = 427 \text{ LB/BOLT}$$

$$T_U \text{ MAX} = 293\# + 157\#(0.3) + 427\# = 767 \text{ LB/BOLT (MAX)}$$

SHEAR (V)

$$V_U \text{ WALL} = \sqrt{\left(\frac{(1.2(1600\#) + 640\#)}{18 \text{ BOLTS}}\right)^2 + \left(\frac{7680\#}{18 \text{ BOLTS}}\right)^2} = 450 \text{ LB/BOLT (MAX)}$$

#### INTERACTION:

$$\left(\frac{T_u}{\phi T}\right) + \left(\frac{V_u}{\phi V}\right) \leq 1.2$$

$$\left(\frac{767}{1586}\right) + \left(\frac{450}{2201}\right) = 0.69 \leq 1.2 \therefore \text{O.K.}$$

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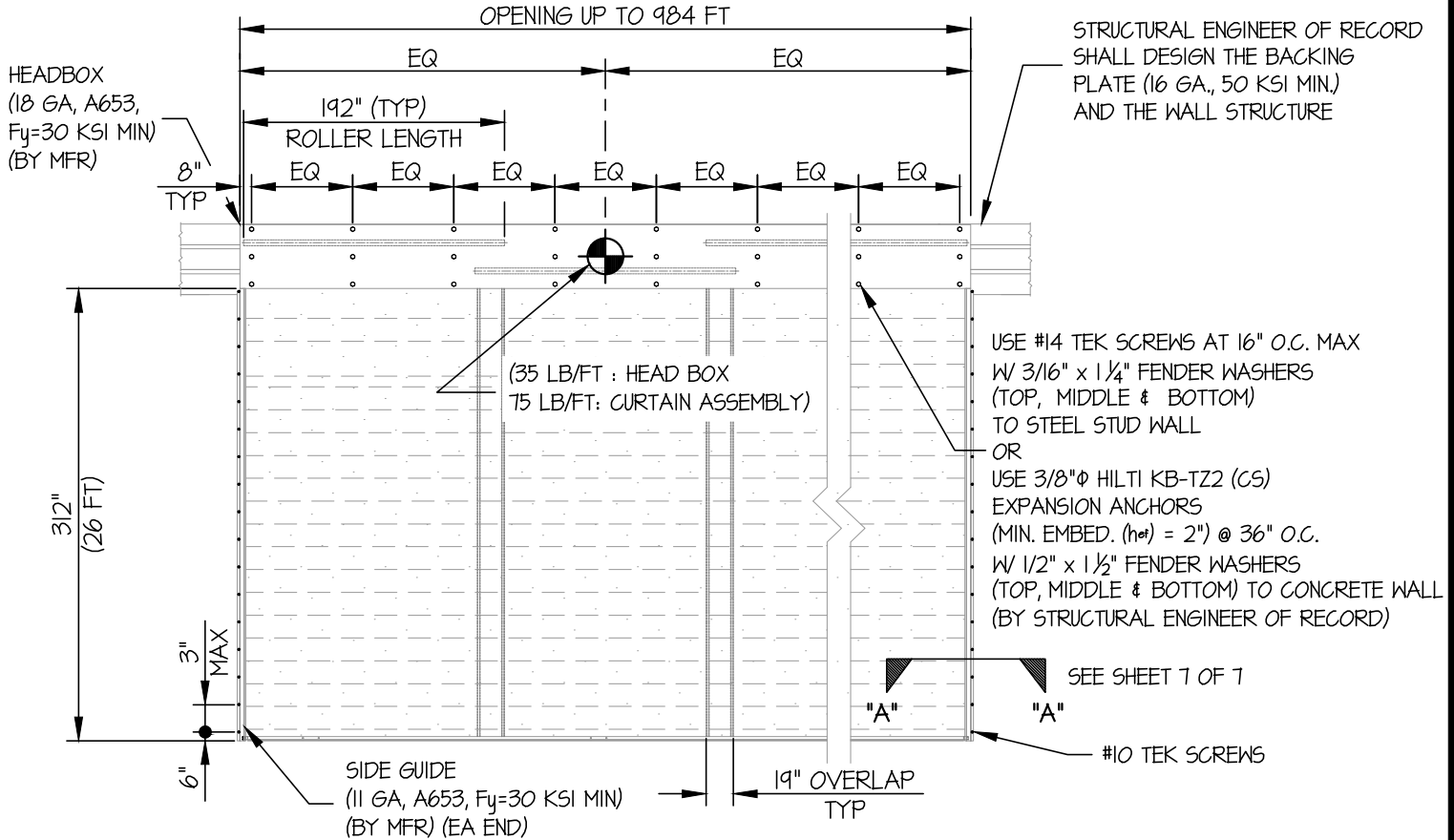
SHEET

4

OF 7 SHEETS

### SEISMIC SUPPORTS & ATTACHMENTS

### WALL MOUNTED



### ELEVATION (MULTIPLE CURTAINS)

#### NOTES:

- FORCES ARE DETERMINED PER 2022 CALIFORNIA BUILDING CODE AND ASCE 7-16. STRENGTH DESIGN IS USED. (EXAMPLE:  $a_p = 1.0$ ,  $l_p = 1.5$ ,  $R_p = 1.5$ ,  $z/h \leq 1$ )

Sds	1.60	2.00
HORIZONTAL FORCE ( $E_h$ )	1.92 $W_p$	2.40 $W_p$
HORIZONTAL FORCE ( $E_{mh}$ )	3.84 $W_p$	4.80 $W_p$
VERTICAL FORCE ( $E_v$ )	0.32 $W_p$	0.40 $W_p$

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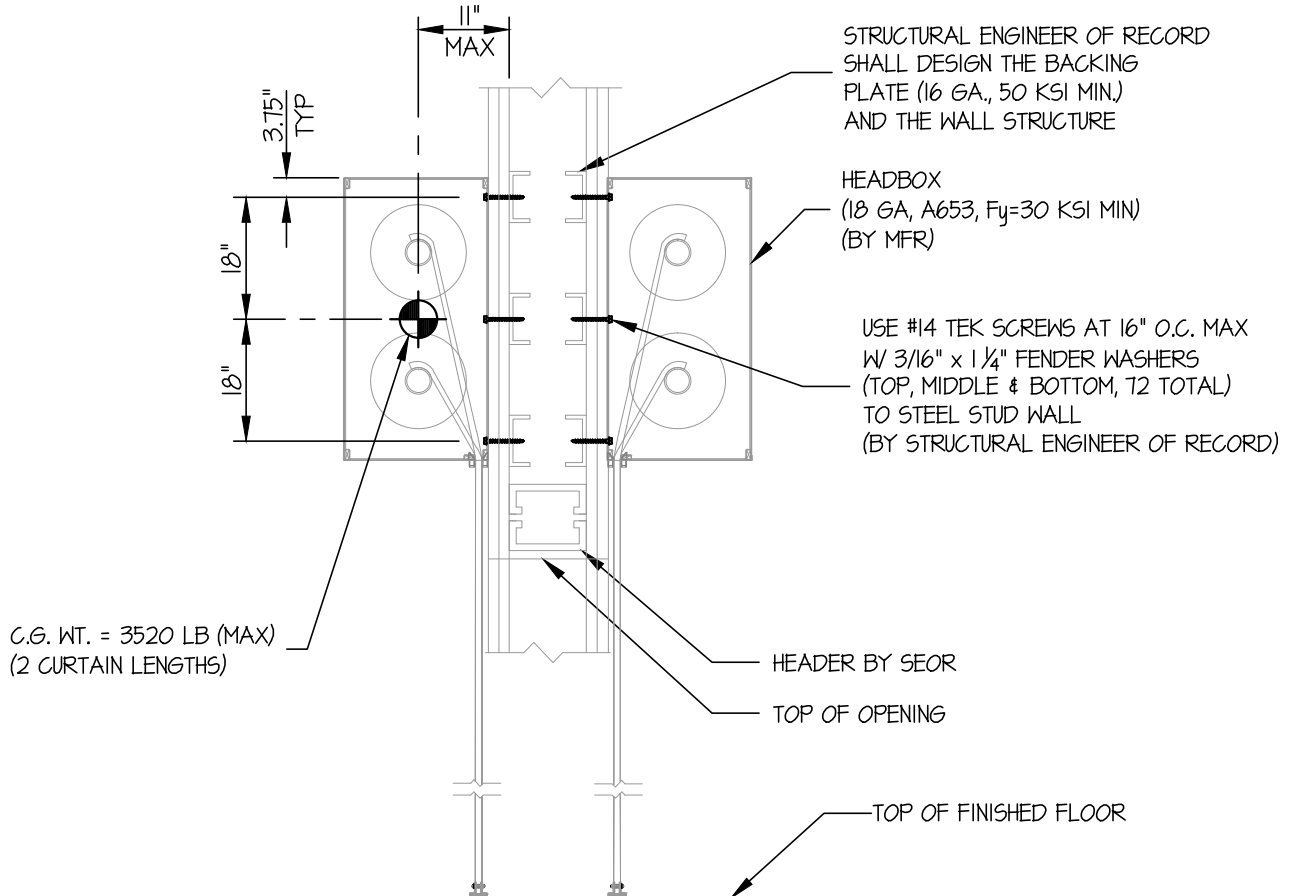
5

OF 7 SHEETS

SEISMIC SUPPORTS & ATTACHMENTS

$S_{DS} \leq 1.60$

WALL MOUNTED



#### SECTION AT STEEL STUD WALL (MULTIPLE CURTAINS)

##### LOADS:

WEIGHT ( $W_p$ ) = 3520 LB MAX (2 CURTAIN LENGTHS)  
HORIZONTAL FORCE ( $E_h$ ) = 192  $W_p$  = 6759 LB  
VERTICAL FORCE ( $E_v$ ) = 0.32  $W_p$  = 1127 LB  
(EXAMPLE:  $S_{DS} = 1.60$ ,  $a_p = 1.0$ ,  $l_p = 1.5$ ,  $R_p = 1.5$ ,  $\Omega_o = 2.0$ ,  $z/h = 0$ )

##### SCREW FORCES:

TENSION (T)

$$T_{U \text{ VERTICAL}} = \frac{(12(3520\#) + 1127\#)(11")}{24 \text{ SCREWS}(36")} = 68 \text{ LB/SCREW}$$

$$T_{U \text{ PARALLEL}} = \frac{6759\#(11")}{3 \text{ SCREWS}(36")} = 68 \text{ LB/SCREW}$$

$$T_{U \text{ PERP}} = \frac{6759\#}{72 \text{ SCREWS}} = 94 \text{ LB/SCREW}$$

$$T_{U \text{ MAX}} = 68\# + 68\#(0.3) + 94\# = 183 \text{ LB/SCREW (MAX)}$$

SHEAR (V)

$$V_{U \text{ WALL}} = \sqrt{\left(\frac{(12(3520\#) + 1127\#)}{72 \text{ SCREWS}}\right)^2 + \left(\frac{6759\#}{72 \text{ SCREWS}}\right)^2} = 120 \text{ LB/SCREW (MAX)}$$

#14 TEK SCREWS (16 GA, 50 KSI STEEL STUDS)

W/ 2 LAYERS GYP BOARD MAX

$\phi T = 418 \text{ LB/SCREW}$

$\phi V = 266 \text{ LB/SCREW}$

##### UNITY CHECK:

$$\left(\frac{T_u}{\phi T}\right) + \left(\frac{V_u}{\phi V}\right) \leq 1.0$$

$$\left(\frac{183}{418}\right) + \left(\frac{120}{266}\right) = 0.89 \leq 1.0 \therefore \text{O.K.}$$

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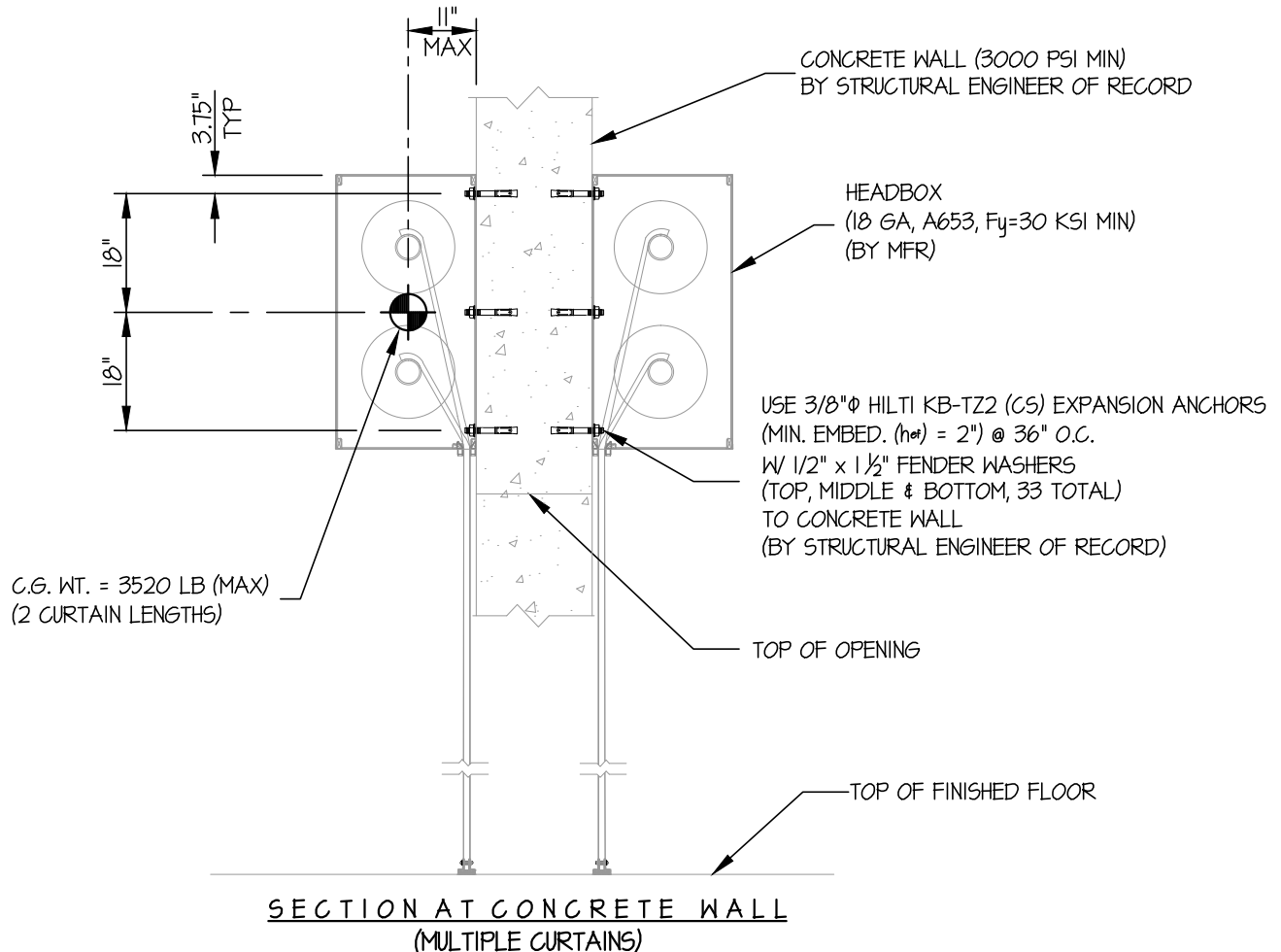
6

OF 7 SHEETS

SEISMIC SUPPORTS & ATTACHMENTS

$S_{DS} \leq 2.00$

WALL MOUNTED



#### LOADS:

WEIGHT ( $W_p$ ) = 3520 LB MAX (2 CURTAIN LENGTHS)  
HORIZONTAL FORCE ( $E_{mh}$ ) = 4.80  $W_p$  = 16,896 LB  
VERTICAL FORCE ( $E_v$ ) = 0.40  $W_p$  = 1408 LB  
(EXAMPLE:  $S_{DS}$  = 2.00,  $a_p$  = 1.0,  $I_p$  = 1.5,  $R_p$  = 1.5,  $\Omega_0$  = 2.0,  $z/h$  = 0)

#### SCREW FORCES:

TENSION (T)

$$T_U \text{ VERTICAL} = \frac{(12(3520\#) + 1408\#)(11")}{11 \text{ BOLTS } (36")} = 157 \text{ LB/BOLT}$$

$$T_U \text{ PARALLEL} = \frac{16896\#(11")}{3 \text{ BOLTS } (36")} = 168 \text{ LB/BOLT}$$

$$T_U \text{ PERP} = \frac{16896\#}{33 \text{ BOLTS}} = 512 \text{ LB/BOLT}$$

$$T_U \text{ MAX} = 157\# + 168\#(0.3) + 512\# = 720 \text{ LB/BOLT (MAX)}$$

SHEAR (V)

$$V_U \text{ WALL} = \sqrt{\left(\frac{(12(3520\#) + 1408\#)}{33 \text{ BOLTS}}\right)^2 + \left(\frac{16896\#}{33 \text{ BOLTS}}\right)^2} = 540 \text{ LB/BOLT (MAX)}$$

ANCHOR SPEC: 3/8"  $\phi$  HILTI KB-TZ2 (CS); ( $h_{ef}$  = 2")  
SPACING = 6" MIN  
EDGE DISTANCE = 14" MIN:  
 $\phi T = 0.75 \phi N_h = 1586 \text{ LB/ANCHOR (TENSION)}$   
 $\phi V = \phi V_h = 2201 \text{ LB/ANCHOR (SHEAR)}$

#### INTERACTION:

$$\left(\frac{T_u}{\phi T}\right) + \left(\frac{V_u}{\phi V}\right) \leq 1.2$$

$$\left(\frac{720}{1586}\right) + \left(\frac{540}{2201}\right) = 0.70 \leq 1.2 \therefore \text{O.K.}$$

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7

OF 7 SHEETS

SEISMIC SUPPORTS & ATTACHMENTS

WALL MOUNTED

STRUCTURAL ENGINEER OF RECORD  
SHALL DESIGN THE BACKING  
PLATE (16 GA., 50 KSI MIN.)  
AND THE WALL STRUCTURE

USE #14 TEK SCREWS  
W/ STANDARD WASHERS  
TO STEEL STUD WALL  
(BY STRUCTURAL ENGINEER OF RECORD)

5/8" THK  
WALL BOARD  
(2 LAYER MAX)

EDGE OF  
OPENING

L 3 x 2 (11 GA, A653)  
W/ #4 x 3/8" L RIVET  
(BY MFR)

SIDE GUIDE  
(11 GA, A653, Fy=30 KSI MIN)  
(BY MFR) (2 TOTAL)

L 3 x 2 (11 GA, A653)  
W/ #4 x 3/8" L RIVET  
(BY MFR)

SECTION AT STEEL STUD WALL

USE 3/8"φ HILTI KB-TZ2 (CS)  
EXPANSION ANCHORS  
(MIN. EMBED. (h<sub>ef</sub>) = 2")  
W/ STANDARD WASHERS  
TO CONCRETE WALL  
(BY STRUCTURAL ENGINEER  
OF RECORD)

EDGE OF  
OPENING

L 3 x 2 (11 GA, A653)  
W/ #4 x 3/8" L RIVET  
(BY MFR)

SIDE GUIDE  
(11 GA, A653, Fy=30 KSI MIN)  
(BY MFR) (2 TOTAL)

L 3 x 2 (11 GA, A653)  
W/ #4 x 3/8" L RIVET  
(BY MFR)

SECTION AT CONCRETE WALL

SECTION A-A