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DOOR SYSTEMS INC

DSI-HS10B HOSE STREAM RATED SMOKE & FIRE CURTAIN

DES. J. ROBERSON

ЈОВ NO. 11-2404

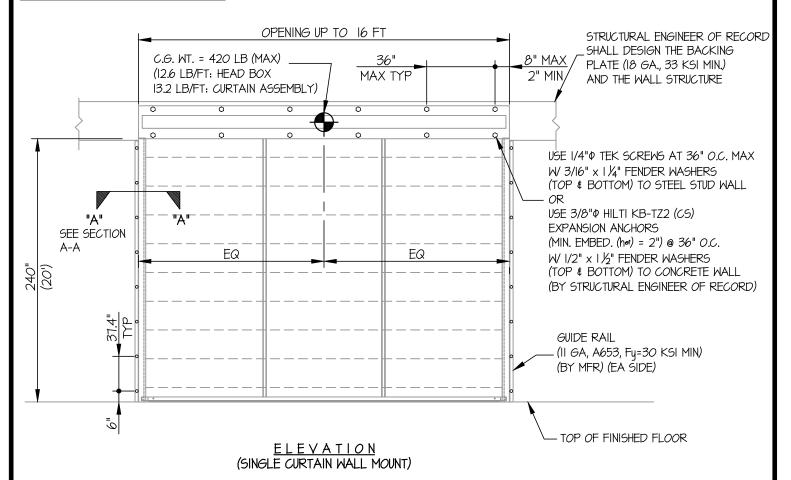
DATE 8/28/24

1

- 14 _{SHEETS}

SEISMIC SUPPORTS & ATTACHMENTS

MALL MOUNTED



NOTES:

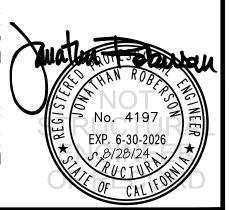
1. FORCES ARE DETERMINED PER 2022 CALIFORNIA BUILDING CODE AND ASCE 7-16. STRENGTH DESIGN IS USED. (EXAMPLE: SDS = 2.00, 20

HORIZONTAL FORCE (En) = 2.40 Wp

HORIZONTAL FORCE (Emh) = 4.80 Wp (FOR CONCRETE ANCHORAGE)

VERTICAL FORCE (Ev) = 0.40 Wp

- 2. THIS PREAPPROVA ENCOMPASSES WEIGHTS AND VERTICAL C.G. POSITIONS NOT EXCEEDING VALUES SHOWN.
- 3. THIS PREAPPROVA WAS PREPARED WITHOUT KNOWLEDGE OF ANY SITE CONDITION. COMPATIBILITY FOR USE WITH A SITE SHALL BE EVALUATED BY THE STRUCTURAL ENGINEER OF RECORD OF THE INSTALLATION (SEOR). USE REQUIRES APPROVAL BY THE SEOR.
- 4. STRUCTURAL ENGINEER OF RECORD FOR THE INSTALLATION SHALL VERIFY ALL CONDITIONS, EVALUATE INTERACTION WITH ADJACENT EQUIPMENT AND ANCHORS, AND PROVIDE SUPPORT STRUCTURE DESIGNED TO SUPPORT WEIGHTS AND FORCES SHOWN IN COMBINATION WITH ALL OTHER LOADS THAT MAY BE PRESENT.



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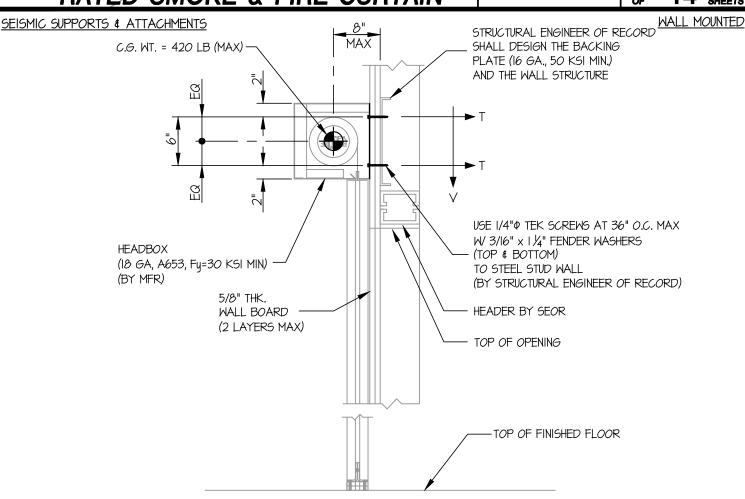
DSI-HS10B HOSE STREAM RATED SMOKE & FIRE CURTAIN

DER. J. ROBERSON

11-2404 JOB NO.

8/28/24 DATE

SHEETS



SECTION AT STEEL STUD WALL (SINGLE CURTAIN WALL MOUNT)

LOADS:

= 420 LB MAX WEIGHT (Wp)

HORIZONTAL FORCE (Eh) = 2.40 Wp = 1008 LB

VERTICAL FORCE (Ev) = 0.40 Wp = 168 LB

SCREW FORCES:

TENSION (T)

Tu vertical = $\frac{(1.2(420\#) + 168\#)(8")}{(1.2(420\#) + 168\#)(8")} = 150 LB/SCREW$ 6 SCREWS (6")

Tu parallel = $\frac{1008\#(8")}{2\text{screws}(180")}$ = 23 LB/SCREW

 $= \frac{1008\#}{12 \text{ SCREW}} = 84 \text{ LB/SCREW}$

Tu max = 150# + 23#(0.3) + 84# = 241 LB/SCREW (MAX)

SHEAR (V)

Tu PERP

= 101 LB/SCREW (MAX) Vu wall

1/4"ø TEK SCREWS (16 GA, 50 KSI STEEL STUDS)

W/ 2 LAYERS GYP BOARD MAX

ΦT = 418 LB/SCREW

ΦV= 266 LB/SCREW

INTERACTION

$$\left(\frac{\mathsf{Tu}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{Vu}}{\mathsf{\Phi}\mathsf{V}}\right) \le 1.0$$

$$\left(\frac{241}{418}\right) + \left(\frac{101}{266}\right) = 0.96 \le 10$$
 ... OK.

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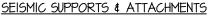
DES. J. ROBERSON

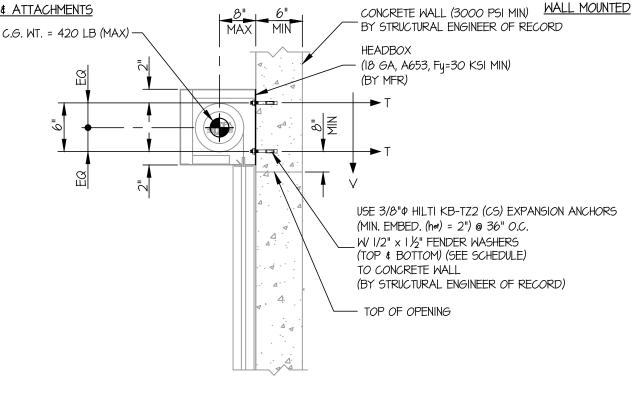
JOB NO. 11-2404

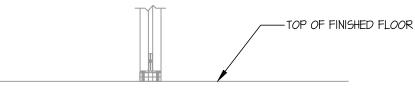
DATE 8/28/24

3

14 SHEETS







SECTION AT CONCRETE WALL

(SINGLE ROLLER WALL MOUNT)

LOADS:

WEIGHT (Wp) = 420 LB MAX HORIZONTAL FORCE (En) = 4.80 Wp = 2016 LB VERTICAL FORCE (Ev) = 0.40 Wp = 168 LB

ANCHOR FORCES:

TENSION (T)

Tu vertical =
$$\frac{(1.2(420\#) + 168\#)(8")}{6 \text{ BOLTS}} = 150 \text{ LB/BOLT}$$

Tu parallel =
$$\frac{2016\#(8'')}{2\text{Bolts} (180'')}$$
 = 45 LB/BOLT

Tu perp =
$$\frac{2016\#}{12 \text{ BoLTs}}$$
 = 168 LB/BOLT

Tu max = 150# + 45#(0.3) + 168# = 332 LB/BOLT (MAX)

SHEAR (V)

Vu wall =
$$\sqrt{\left(\frac{(1.2(420\#) + 168\#)}{12 \text{ BOLTS}}\right)^2 + \left(\frac{2016\#}{12 \text{ BOLTS}}\right)^2} = 177 \text{ LB/BOLT (MAX)}$$

ANCHOR SPEC: 1/2" P HILTI KB-TZ2 (CS): (hef = 2")

SPACING = 6" MIN EDGE DISTANCE = 8" MIN:

 $\phi T = 0.75 \phi Nn$ = 1586 LB/ANCHOR (TENSION) $\phi V = \phi Vn$ = 1463 LB/ANCHOR (SHEAR)

INTERACTION:

$$\left(\frac{\mathsf{Tu}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{Vu}}{\mathsf{\Phi}\mathsf{V}}\right) \le 1.0$$

$$\left(\frac{332}{1586}\right) + \left(\frac{177}{1463}\right) = 0.33 \le 10$$
 .°. O.K.

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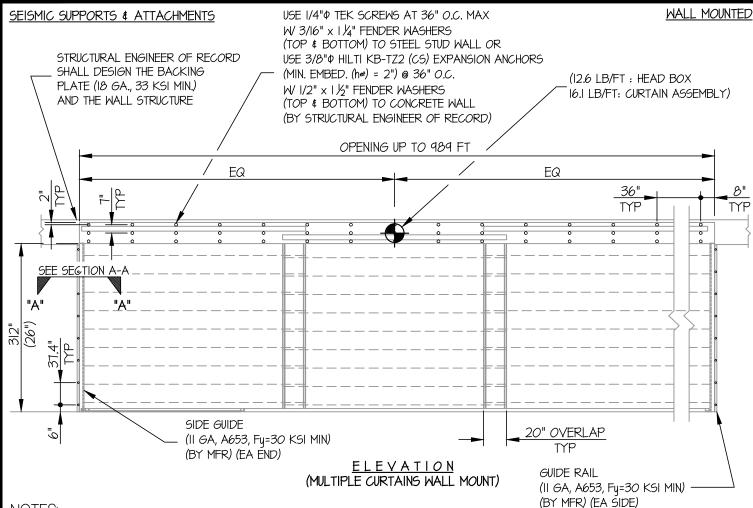
ЈОВ NO. 11-2404

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of 14 SHEETS



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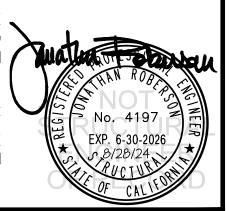
1. FORCES ARE DETERMINED PER 2022 CALIFORNIA BUILDING CODE AND ASCE 7-16. STRENGTH DESIGN IS USED. (EXAMPLE: SDS = 2.30, 20 = 1.0, 10 = 1.5, 10 = 2.0, 10 = 2.0, 10 = 1.5, 10 = 2.0, 10 = 1.5, 10

HORIZONTAL FORCE (Eh) = 2.76 Wp

HORIZONTAL FORCE (Emh) = 5.52 Wp (FOR CONCRETE ANCHORAGE)

VERTICAL FORCE (Ev) = 0.46 Wp

- 2. THIS PREAPPROVA ENCOMPASSES WEIGHTS AND VERTICAL C.G. POSITIONS NOT EXCEEDING VALUES SHOWN.
- 3. THIS PREAPPROVA WAS PREPARED WITHOUT KNOWLEDGE OF ANY SITE CONDITION. COMPATIBILITY FOR USE WITH A SITE SHALL BE EVALUATED BY THE STRUCTURAL ENGINEER OF RECORD OF THE INSTALLATION (SEOR). USE REQUIRES APPROVAL BY THE SEOR.
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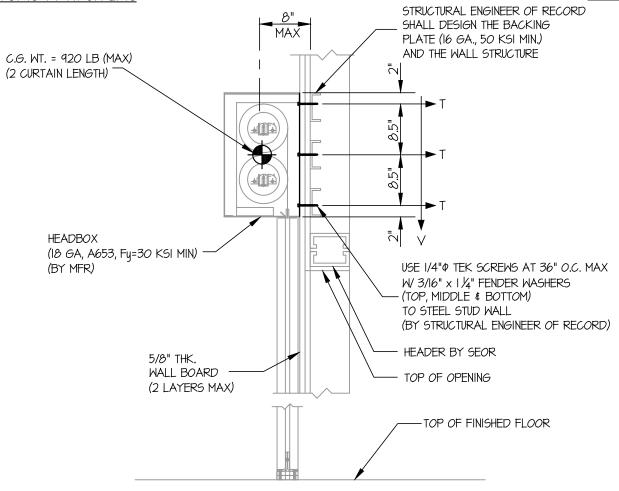
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F 14 SHEETS

SEISMIC SUPPORTS & ATTACHMENTS

MALL MOUNTED



SECTION AT STEEL STUD WALL (MULTIPLE CURTAINS WALL MOUNT)

LOADS:

WEIGHT (Wp) = 920 LB MAX (2 CURTAIN LENGTHS)

HORIZONTAL FORCE (En) = 2.76 Wp = 2539 LB VERTICAL FORCE (Ev) = 0.46 Wp = 423 LB

SCREW FORCES: TENSION (T)

Tu vertical = $\frac{(1.2(920\#) + 423\#)(8")}{12\cos(17")} = 60 \text{ LB/SCREW}$

Tu parallel = $\frac{2539\#(8'')}{3 \text{ screws}(180'')}$ = 38 LB/SCREW

Tu perp = $\frac{2539\#}{33 \text{ screws}}$ = 77 LB/SCREW

Tu max = 60# + 38#(0.3) + 77# = 149 LB/SCREW (MAX)

SHEAR (V)

Vu wall = $\sqrt{\left(\frac{(1.2(920\#) + 423\#)}{33 \text{ screws}}\right)^2 + \left(\frac{2539\#}{33 \text{ screws}}\right)^2} = 90 \text{ LB/SCREW (MAX)}$

1/4"¢ TEK SCREWS (16 GA, 50 KSI STEEL STUDS)

W/ 2 LAYERS GYP BOARD MAX \$\phi T = 418 LB/SCREW

\$\phi V = 266 LB/SCREW\$

INTERACTION:

$$\left(\frac{\mathsf{Tu}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{Vu}}{\mathsf{\Phi}\mathsf{V}}\right) \le 1.0$$

$$\left(\frac{149}{418}\right) + \left(\frac{90}{266}\right) = 0.70 \le 1.0$$
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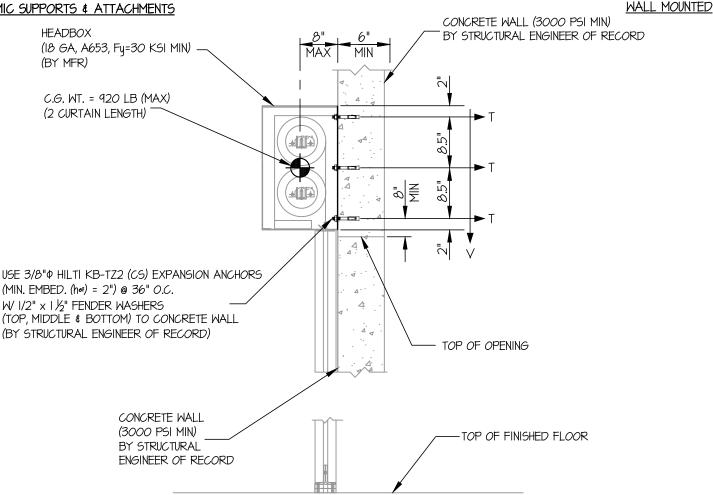
11-2404 JOB NO.

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SEISMIC SUPPORTS & ATTACHMENTS



SECTION AT CONCRETE WALL (MULTIPLE CURTAINS WALL MOUNT)

LOADS:

= 920 LB (2 CURTAIN LENGTHS) WEIGHT (Wp)

HORIZONTAL FORCE (Eh) = 5.52 Wp = 5079 LB VERTICAL FORCE (Ev) = 0.46 Wp = 423 LB

ANCHOR FORCES:

TENSION (T)

$$\frac{(1.2(920\#) + 423\#)(8")}{42.444} = 60 LB/BOLT$$

Tu vertical =

$$Tu parallel = \frac{5079\#(8")}{3 \text{ Bolts (180")}} = 75 \text{ LB/BOLT}$$

$$\frac{5079\#}{33\,\text{ROLTS}}$$
 = 154 LB/BOLT

Tu PERP 33 BOLTS

Tu
$$MAX$$
 = 60# + 75#(0.3) + 154# = 237 LB/BOLT (MAX)

SHEAR (V)

Vu wall =
$$\sqrt{\frac{(12(920\#) + 423\#)}{33 \text{ BoLTS}}} + (\frac{5079\#}{33 \text{ BoLTS}})^2 = 161 \text{ LB/BOLT (MAX)}$$

ANCHOR SPEC: 3/8" HILTI KB-TZ2 (CS): (hef = 2")

SPACING = 6" MIN EDGE DISTANCE = 8" MIN:

 $\phi T = 0.75 \phi Nn$ = 1586 LB/ANCHOR (TENSION) = 1463 LB/ANCHOR (SHEAR) $\Phi V = \Phi V n$

INTERACTION:

$$\left(\frac{\mathsf{T}_{\mathsf{u}}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{V}_{\mathsf{u}}}{\mathsf{\Phi}\mathsf{V}}\right) \le 1.2$$

$$\left(\frac{237}{1586}\right) + \left(\frac{161}{1463}\right) = 0.26 \le 1.2$$
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SEISMIC SUPPORTS & ATTACHMENTS

WALL MOUNTED OPENING UP TO 16 FT C.G. WT. = 420 LB (MAX) 8" MAX (I2.6 LB/FT : HEAD BOX 2" MIN 13.2 LB/FT: CURTAIN ASSEMBLY) USE 1/4" TEK SCREWS AT 24" O.C. MAX W/3/16" x 1 1/4" FENDER WASHERS "B" "B" (EACH SIDE) TO STEEL STUD WALL SEE SECTION (BY STRUCTURAL ENGINEER OF RECORD) (20) GUIDE RAIL (II GA, A653, Fy=30 KSI MIN) (BY MFR) (EA SIDE) - TOP OF FINISHED FLOOR ELEVATION

NOTES:

 FORCES ARE DETERMINED PER 2022 CALIFORNIA BUILDING CODE AND ASCE 7-16. STRENGTH DESIGN IS USED. (EXAMPLE: SDS = 2.00, Δp = 1.0, lp = 1.5, Rp = 1.5, Ω_0 = 2.0, z/h < 1)

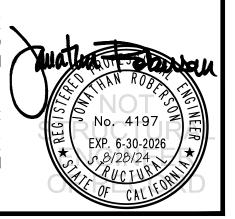
(SINGLE ROLLER CEILING MOUNT)

HORIZONTAL FORCE (En) = 2.40 Wp

HORIZONTAL FORCE (Emh) = 4.80 W_D (FOR CONCRETE ANCHORAGE)

VERTICAL FORCE (Ev) = 0.40 Wp

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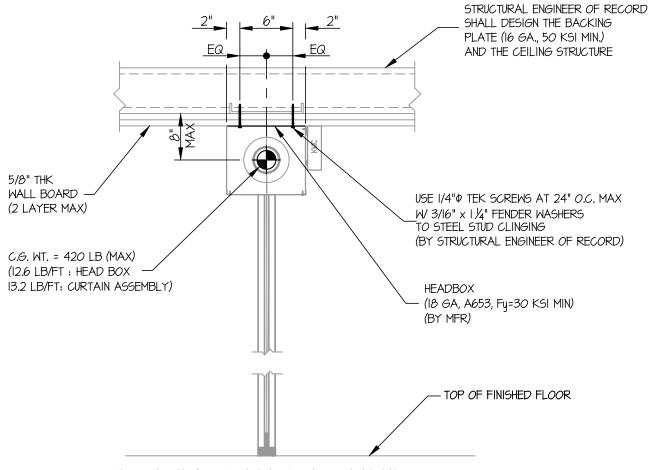
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SHEET

14 _{SHEETS}

SEISMIC SUPPORTS & ATTACHMENTS

CEILING MOUNTED



SECTION AT STEEL STUD CEILING (SINGLE ROLLER CEILING MOUNT)

LOADS:

WEIGHT (W_p) = 420 LB MAX HORIZONTAL FORCE (En) = 2.40 W_p = 1008 LB VERTICAL FORCE (Ev) = 0.40 W_p = 168 LB

SCREW FORCES:

TENSION (T)

Tu vertical =
$$\frac{(1.2(420\#) + 168\#)}{16 \text{ screws}}$$
 = 42 LB/SCREW

Tu parallel =
$$\frac{1008\#(8")}{2 \text{ screws}(180")}$$
 = 23 LB/SCREW

Tu perp =
$$\frac{1008\#(8'')}{8808988(6'')}$$
 = 192 LB/SCREW

Tu
$$\max$$
 = 42# + 23#(0.3) + 192# = 241 LB/SCREW (MAX)

SHEAR (V)

Vu wall =
$$\sqrt{\frac{(1008\#)}{16 \text{ screws}}}^2 + \frac{(1008\#)}{16 \text{ screws}}^2 = 89 \text{ LB/BOLT (MAX)}$$

1/4"¢ TEK SCREWS (16 GA, 50 KSI STEEL STUDS)

W/ 2 LAYERS GYP BOARD MAX

 $\phi T = 418 LB/SCREW$ $\phi V = 266 LB/SCREW$

$$\left(\frac{\mathsf{Tu}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{Vu}}{\mathsf{\Phi}\mathsf{V}}\right) \le 1.0$$

$$\left(\frac{241}{418}\right) + \left(\frac{89}{266}\right) = 0.91 \le 1.0$$
 °. O.K.

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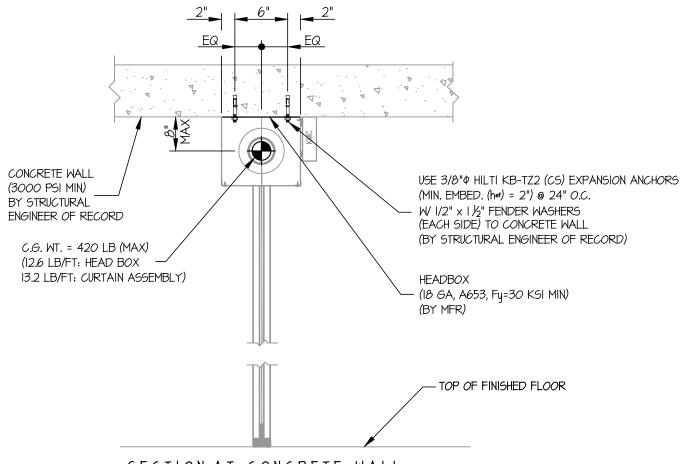
DATE 8/28/24

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F 14 SHEETS

SEISMIC SUPPORTS & ATTACHMENTS

CEILING MOUNTED



SECTION AT CONCRETE WALL (SINGLE ROLLER CEILING MOUNT)

LOADS:

WEIGHT (Wp) = 420 LB MAX HORIZONTAL FORCE (En) = 4.80 Wp = 2016 LB VERTICAL FORCE (Ev) = 0.40 Wp = 168 LB

ANCHOR FORCES:

TENSION (T)

Tu vertical =
$$\frac{(1.2(420\#) + 168\#)}{16 \text{ screens}}$$
 = 42 LB/SCREW

Tu parallel =
$$\frac{2016\#(8'')}{2 \text{ screws}(180'')}$$
 = 45 LB/SCREW

Tu perp =
$$\frac{2016\#(8")}{8 \text{scrpws}(6")}$$
 = 384 LB/SCREW

Tu
$$\max$$
 = 42# + 45#(0.3) + 384# = 402 LB/SCREW (MAX)

SHEAR (V)

Vu wall =
$$\sqrt{\left(\frac{(2016\#)}{16 \text{ screws}}\right)^2 + \left(\frac{(2016\#)}{16 \text{ screws}}\right)^2} = 179 \text{ LB/BOLT (MAX)}$$

ANCHOR SPEC: 3/8" P HILTI KB-TZ2 (CS): (hef = 2")

SPACING = 6" MIN EDGE DISTANCE = 8" MIN:

 ϕ T = 0.75 ϕ Nn = 1586 LB/ANCHOR (TENSION) ϕ V = ϕ Vn = 1463 LB/ANCHOR (SHEAR)

$$\left(\frac{\mathsf{Tu}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{Vu}}{\mathsf{\Phi}\mathsf{V}}\right) \le 1.2$$

$$\left(\frac{402}{1586}\right) + \left(\frac{179}{1463}\right) = 0.38 \le 12$$
 ... O.K.

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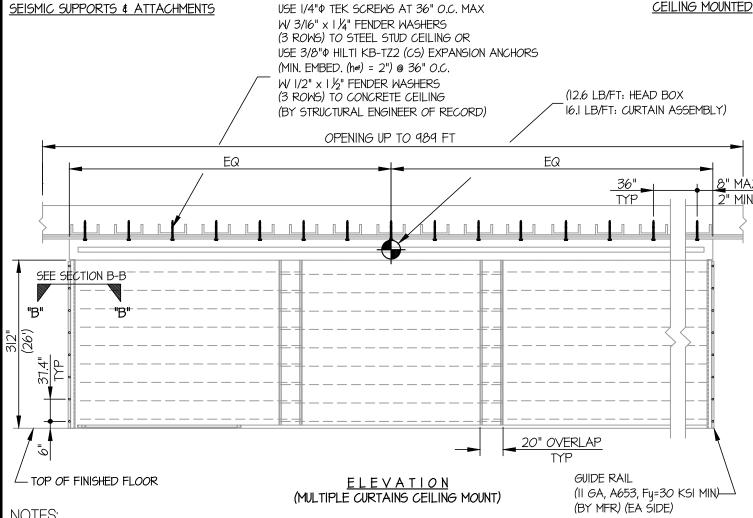
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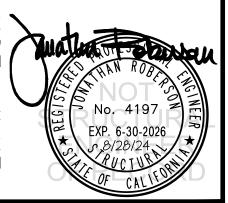
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> HORIZONTAL FORCE (Eh) = 2.76 Wp

HORIZONTAL FORCE (Emh) = 5.52 Wp (FOR CONCRETE ANCHORAGE)

VERTICAL FORCE (Ev) = 0.46 Wp

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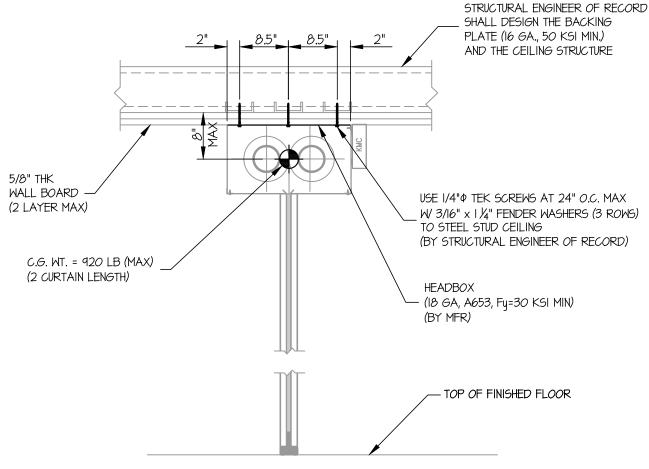
DATE 8/28/24

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_F 14 _{SHEETS}

SEISMIC SUPPORTS & ATTACHMENTS

CEILING MOUNTED



SECTION AT STEEL STUD CEILING (MULTIPLE CURTAINS CEILING MOUNT)

LOADS:

WEIGHT (W_D) = 920 LB (2 CURTAIN LENGTHS)

HORIZONTAL FORCE (E_h) = 2.76 W_p = 2540 LB VERTICAL FORCE (E_v) = 0.46 W_p = 423 LB

SCREW FORCES:

TENSION (T)

Tu vertical =
$$\frac{(1.2(920\#) + 423\#)}{30 \text{ screws}} = 51 \text{ LB/SCREW}$$

Tu parallel =
$$\frac{2540\#(8")}{3\text{screws}(360")}$$
 = 19 LB/SCREW

Tu perp =
$$\frac{2540\#(8'')}{10 \text{ screws}(17'')}$$
 = 120 LB/SCREW

Tu \max = 51# + 19#(0.3) + 120# = 177 LB/SCREW (MAX)

SHEAR (V)

Vu wall =
$$\sqrt{\left(\frac{(2540\#)}{30 \text{ sorews}}\right)^2 + \left(\frac{(2540\#)}{30 \text{ sorews}}\right)^2} = 120 \text{ LB/BOLT (MAX)}$$

1/4"¢ TEK SCREWS (16 GA, 50 KSI STEEL STUDS)

W/ 2 LAYERS GYP BOARD MAX

 ϕ T = 418 LB/SCREW ϕ V= 266 LB/SCREW

$$\left(\frac{\mathsf{Tu}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{Vu}}{\mathsf{\Phi}\mathsf{V}}\right) \le 1.0$$

$$\left(\frac{177}{418}\right) + \left(\frac{120}{266}\right) = 0.88 \le 1.0$$
 ... O.K.

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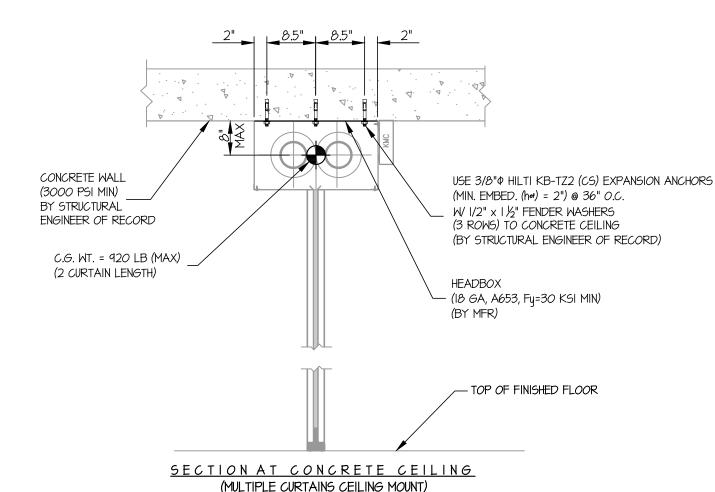
DATE 8/28/24

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F 14 SHEETS

SEISMIC SUPPORTS & ATTACHMENTS

CEILING MOUNTED



LOADS:

WEIGHT (Wp) = 920 LB (2 CURTAIN LENGTHS)

HORIZONTAL FORCE (En) = 5.52 Wp = 5080 LB VERTICAL FORCE (Ev) = 0.46 Wp = 423 LB

ANCHOR FORCES:

TENSION (T)

Tu vertical =
$$\frac{(1.2(920\#) + 423\#)}{30 \text{ pouts}}$$
 = 51 LB/BOLT

Tu parallel =
$$\frac{5080\#(8")}{3 \text{ bolts}(360")}$$
 = 38 LB/BOLT

Tu perp =
$$\frac{5080\#(8'')}{10 \text{ Bol TS}(17'')}$$
 = 239 LB/BOLT

Tu
$$\max$$
 = 51# + 38#(0.3) + 239# = 302 LB/BOLT (MAX)

SHEAR (V)

Vu wall =
$$\sqrt{\left(\frac{5080\#}{30 \text{ Bolts}}\right)^2 + \left(\frac{5080\#}{30 \text{ Bolts}}\right)^2} = 240 \text{ LB/BOLT (MAX)}$$

ANCHOR SPEC: 3/8" P HILTI KB-TZ2 (CS); (hef = 2")

SPACING = 6" MIN EDGE DISTANCE = 8" MIN:

 ϕ T = 0.75 ϕ Nn = 1586 LB/ANCHOR (TENSION) ϕ V = ϕ Vn = 1463 LB/ANCHOR (SHEAR)

$$\left(\frac{\mathsf{Tu}}{\mathsf{\Phi}\mathsf{T}}\right) + \left(\frac{\mathsf{Vu}}{\mathsf{\Phi}\mathsf{V}}\right) \le 1.2$$

$$\left(\frac{302}{1586}\right) + \left(\frac{240}{1463}\right) = 0.37 \le 1.2$$
 ... O.K.

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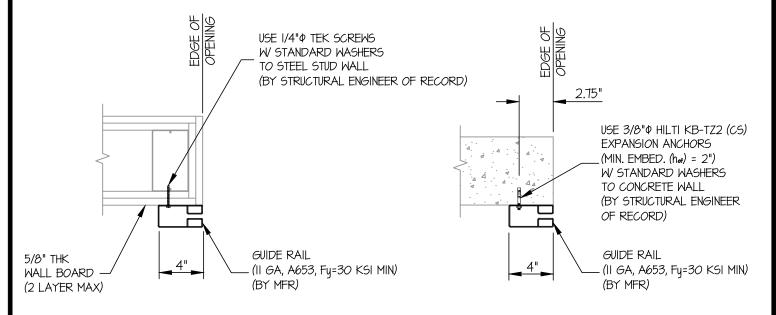
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F 14 SHEETS

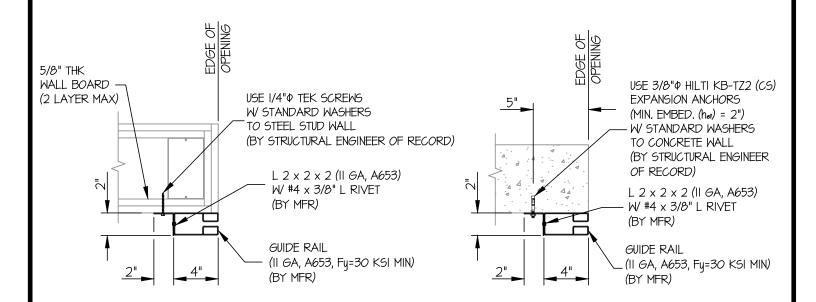
SEISMIC SUPPORTS & ATTACHMENTS

<u>WALL MOUNTED</u>



SECTION AT STEEL STUD WALL

SECTION AT CONCRETE WALL



GUIDE RAIL WITH ANGLE FACE FIX ON STUD WALL (OPTION)

GUIDE RAIL WITH ANGLE FACE FIX ON CONCRETE WALL (OPTION)

SECTION A-A

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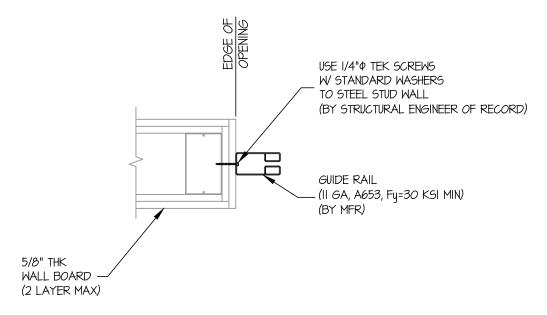
DATE 8/28/24

14

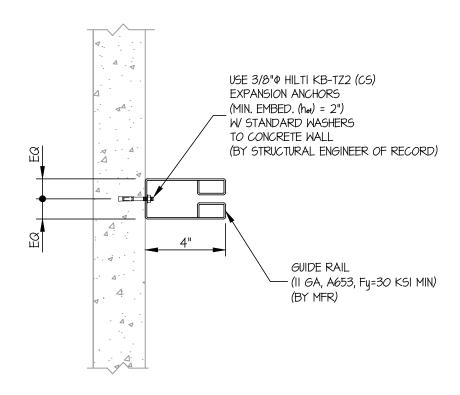
of 14 SHEETS

SEISMIC SUPPORTS & ATTACHMENTS

CEILING MOUNTED



SECTION AT STEEL STUD WALL



SECTION AT CONCRETE WALL

SECTION B-B